

Ultimate bearing capacity calculation according to EC 7, Annex D - RECTANGULAR FOUNDATION

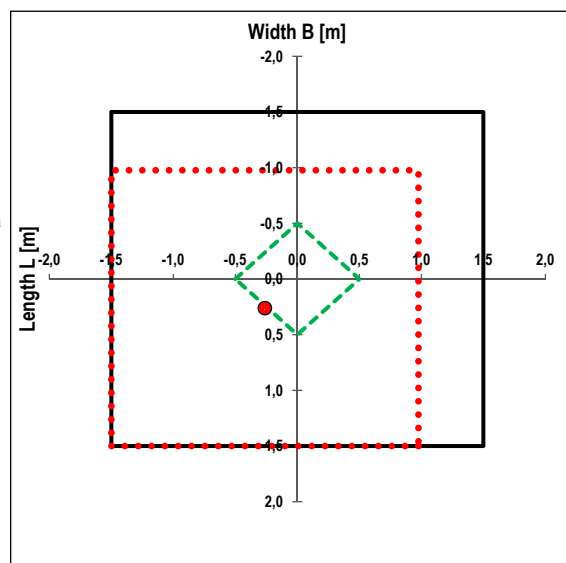
Project: Nosilnost temeljnih tal pod točkovnim temeljem

Conditions: ☒ Drained conditions ☐ Undrained conditions

Foundation dimensions:

$B = 3,00$ m ...foundation width
 $L = 3,00$ m ...foundation length
 $T = 1,00$ m ...foundation thickness
 $z = 1,00$ m ...depth of foundation below the surface
 $\alpha = 0,00$ ° ...inclination of foundation base

— Foundation
 - - - Kern
 • Resulting vertical force
 Effective foundation area



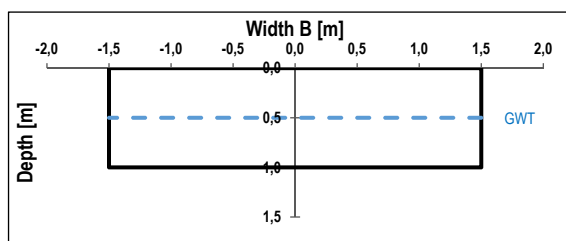
Soil parameters:

$\gamma = 20,00$ kN/m³ ...unit weight of soil below foundation base
 $\varphi' = 30,00$ ° ...effective angle of internal friction of soil
 $c' = 0,00$ kPa ...effective cohesion of soil

GWT present:

☐ No ☒ Yes

$h_w = 0,50$ m ...depth of water below the surface



Effective soil parameters and GWT conditions:

$\gamma_{sat} = 20,00$ kN/m³ ...saturated unit weight of soil
 $\gamma_w = 10,00$ kN/m³ ...unit weight of water

$$\gamma' = \gamma_{sat} - \gamma_w$$

$\gamma' = 10,00$ kN/m³ ...effective unit weight of soil below foundation base

GWT conditions: $0 \leq h_w \leq z \rightarrow$
 $z < h_w \leq z + B(L) \rightarrow$
 $h_w > z + B(L) \rightarrow$

$\bar{\gamma} = \gamma'$...water table is located above foundation base
 $\bar{\gamma} = \gamma' + \frac{(h_w - z)}{B(L)}(\gamma - \gamma')$...water table is located below foundation base
 $\bar{\gamma} = \gamma$...water has no effect on bearing capacity

$\bar{\gamma} = 10,00$ kN/m³ ...average effective unit weight of soil below foundation base

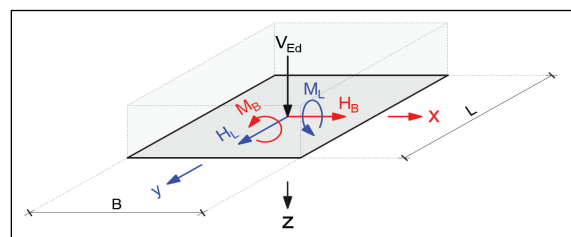
$q' = 15,00$ kPa ...effective compression at foundation level

$F_U = -45,00$ kN ...uplift on the foundation

use: ☐ No ☒ Yes

Design load on foundation base:

$V_{Ed} = 1000,00$ kN ...vertical force (compression is positive)
 $M_B = 250,00$ kNm ...bending moment in B direction
 $M_L = 250,00$ kNm ...bending moment in L direction
 $H_B = 50,00$ kN ...horizontal force in B direction
 $H_L = 50,00$ kN ...horizontal force in L direction



Eccentricities:

$$e_B = \frac{M_B}{V_{Ed}}$$

$e_B = -0,262$ m

...eccentricity of axial force in B direction

$$\frac{e_B}{B} \leq \frac{1}{3}$$



$$e_L = \frac{M_L}{V_{Ed}}$$

$e_L = 0,262$ m

...eccentricity of axial force in L direction

$$\frac{e_L}{L} \leq \frac{1}{3}$$



$$e_T = \sqrt{\left(\frac{e_B}{B}\right)^2 + \left(\frac{e_L}{L}\right)^2}$$

$e_T = 0,123$

...dimensionless total eccentricity

$$e_T \leq \frac{1}{3}$$



Effective foundation dimensions:

$$B' = B - 2 \cdot e_B$$

$B' = 2,476$ m

...effective foundation width

$$L' = L - 2 \cdot e_L$$

$L' = 2,476$ m

...effective foundation length

$$A' = B' \cdot L'$$

$A' = 6,133$ m²

...effective foundation area

DRAINED CONDITIONS:

Factors for the bearing resistance:

$$N_q = e^{\pi \cdot \tan \varphi'} \cdot \tan^2 \left(45 + \frac{\varphi'}{2} \right) \quad N_q = 18,401 \quad \dots \text{dimensionless factor for loading}$$

$$N_c = (N_q - 1) \cdot \cot \varphi' \quad N_c = 30,140 \quad \dots \text{dimensionless factor for cohesion}$$

$$N_\gamma = 2 \cdot (N_q - 1) \cdot \tan \varphi' \quad N_\gamma = 20,093 \quad \dots \text{dimensionless factor for soil density}$$

Factors for the inclination of foundation base:

$$b_c = b_q - \frac{1 - b_q}{N_c \cdot \tan \varphi'} \quad b_c = 1,000 \quad \dots \text{dimensionless factor for cohesion}$$

$$b_q = (1 - \alpha \cdot \tan \varphi')^2 \quad b_q = 1,000 \quad \dots \text{dimensionless factor for loading}$$

$$b_\gamma = (1 - \alpha \cdot \tan \varphi')^2 \quad b_\gamma = 1,000 \quad \dots \text{dimensionless factor for soil density}$$

Factors for the shape of foundation:

$$s_q = 1 + \left(\frac{B'}{L'} \right) \cdot \sin \varphi' \quad s_q = 1,500 \quad \dots \text{dimensionless factor for loading}$$

$$s_\gamma = 1 - 0.3 \cdot \frac{B'}{L'} \quad s_\gamma = 0,700 \quad \dots \text{dimensionless factor for soil density}$$

$$s_c = \frac{s_q \cdot N_q - 1}{N_q - 1} \quad s_c = 1,529 \quad \dots \text{dimensionless factor for cohesion}$$

Factors for the inclination of the load, caused by a horizontal load H:

$$H = \sqrt{(H_B^2 + H_L^2)} \quad H = 70,711 \text{ kN} \quad \dots \text{total horizontal force}$$

$$i_c = i_q - \frac{1 - i_q}{N_c \cdot \tan \varphi'} \quad i_c = 0,885 \quad \dots \text{dimensionless factor for cohesion}$$

$$i_q = \left[1 - \frac{H}{V + A' \cdot c' \cdot \cot \varphi'} \right]^m \quad i_q = 0,891 \quad \dots \text{dimensionless factor for loading}$$

$$i_\gamma = \left[1 - \frac{H}{V + A' \cdot c' \cdot \cot \varphi'} \right]^{m+1} \quad i_\gamma = 0,825 \quad \dots \text{dimensionless factor for soil density}$$

$$m_B = \frac{2 + \frac{B'}{L'}}{1 + \frac{B'}{L'}} \quad m_B = 1,500 \quad \dots \text{exponent in formulas for the inclination factor i}$$

$$m_L = \frac{2 + \frac{L'}{B'}}{1 + \frac{L'}{B'}} \quad m_L = 1,500 \quad \dots \text{exponent in formulas for the inclination factor i}$$

$$m = m_L \cdot \cos^2 \theta + m_B \cdot \sin^2 \theta \quad m = 1,500 \quad \dots \text{exponent in formulas for the inclination factor i}$$

$$\tan \theta = \frac{H_B}{H_L} \quad \theta = 45,000^\circ \quad \dots \text{inclination of horizontal force forming an angle with L' direction}$$

Design vertical load and design vertical bearing resistance:

$$V_d = \frac{V_{Ed}}{A'} \quad V_{Ed}/A' = 155,72 \text{ kPa} \quad \dots \text{design vertical load of foundation base}$$

$$\frac{R}{A'} = c' \cdot N_c \cdot b_c \cdot s_c \cdot i_c + q' \cdot N_q \cdot b_q \cdot s_q \cdot i_q + 0.5 \cdot \bar{\gamma} \cdot B' \cdot N_\gamma \cdot b_\gamma \cdot s_\gamma \cdot i_\gamma$$

$$R_d/A' = 366,14 \text{ kPa} \quad \dots \text{design vertical bearing resistance of foundation base}$$

Bearing resistance utilization:

$$U = 42,53 \% \quad \checkmark \quad \text{BEARING RESISTANCE IS SUFFICIENT!}$$